

Napoli Custom Truss

4480 Allegany road
Little Valley, NY

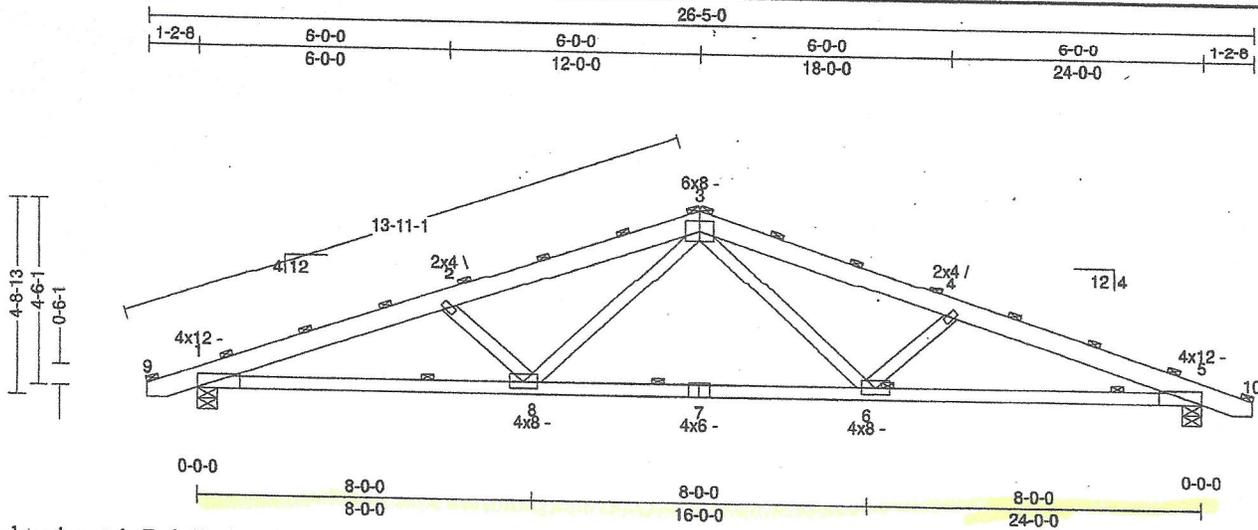
Truss:T01.SBR

Job: NAP0401K-16

Date: 05/28/21 15:10:18

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SPAN 24-0-0	PITCH 4/12	QTY 1	OHL 1-2-8	OHR 1-2-8	CANT L 0-0-0	CANT R 0-0-0	PLYS 1	SPACING 48 in	WGT/PLY 95 lbs
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All plates shown to be Eagle 20 unless otherwise noted.

Loading (psf) TCLL: 29.1 TCDL: 5(rake) BCLL: 0 BCDL: 5	General Bldg Code: IBC 2009/ TP1 1-2007 Rep Mbr: No Lumber D.O.L.: 125 %	CSI TC: 0.94 (3-4) BC: 0.99 (8-1) Web: 0.41 (3-8)	Deflection Vert TL: 0.55 in Vert LL: 0.3 in Horz TL: 0.15 in	L/ L/506 L/913	(loc) (6-7) (6-7) 5	Allowed L/120 L/180
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Reaction

JT	Brg Combo	Brg Width	Rqd Brg Width	Max React	Max Grav Uplift	Max MWFRS Uplift	Max C&C Uplift	Max Uplift	Max Horiz
1	1	5.5 in	3.91 in	2,492 lbs		-880 lbs	-789 lbs	-880 lbs	22 lbs
5	1	5.5 in	3.91 in	2,492 lbs		-880 lbs	-789 lbs	-880 lbs	

Material

TC: SPF#2 2 x 6
BC: SPF 1650/1.5 2 x 4
Web: SPF#2 2 x 4

Bracing

TC: Purlins at 24" OC, Purlin design by Others.
BC: Sheathed or Purlins at 5-6-0, Purlin design by Others.

Loads

- This truss has been designed for the effects of balanced (29.1 psf) and unbalanced sloped roof snow loads in accordance with ASCE7 - 05 with the following user defined input: 55 psf GSL, Terrain C, Exposure (Cc = 1.0), Building Category I (I = 0.80), Thermal (Ct = 1.10), DOL = 1.15. Ventilated. Unobstructed slippery surface. If the roof configuration differs from hip/gable, Building Designer shall verify snow loads.
- This truss has been designed for the effects of wind loads in accordance with ASCE7 - 05 with the following user defined input: 115 mph, Exposure C, Enclosed, Gable/Hip, Building Category I (I = 0.87), h = 15 ft, Not End Zone Truss, Both end webs considered. DOL = 1.60
- This truss has been designed for the effects of TC LL = 20 psf.
- Minimum storage attic loading has not been applied in accordance with IBC 1607.1
- In accordance with IBC 1607.1, minimum BCLL's do not apply.
- This truss is designed as an agricultural truss which for the purposes of this program is defined as a structure that represents a low hazard to people and property. See BCSI-10 for installation and temporary bracing.

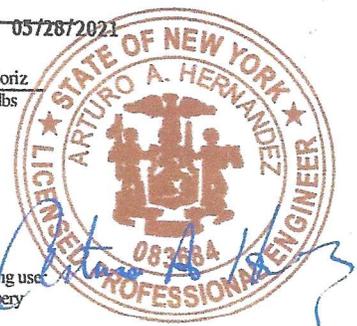
Member Forces

Table indicates: Member ID, max CSI, max axial force, (max comp. force if different from max axial force). Only forces greater than 300lbs are shown in this table.

TC	1-2	0.644	-5,591 lbs	3-4	0.940	-4,622 lbs
	2-3	0.940	-4,622 lbs	4-5	0.644	-5,591 lbs
BC	5-6	0.991	5,173 lbs	6-8	0.674	2,991 lbs
			(-1,603 lbs)	8-1	0.991	5,173 lbs
						(-1,603 lbs)
Web	2-8	0.249	-1,424 lbs	3-8	0.406	1,655 lbs
						(-361 lbs)
				3-6	0.406	1,655 lbs
						(-361 lbs)
				4-6	0.249	-1,424 lbs

Notes

- Unless noted otherwise, do not cut or alter any truss member or plate without prior approval from a Professional Engineer.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed TC DL 5 psf.
- Building Designer shall verify self weight of the truss and other dead load materials do not exceed BCDL 5 psf.
- Design assumes 2x (vertical orientation) purlins are adequately attached to the side of TC & BC therefore sheathing is attached to the TC & BC resulting in a sheathed condition.
- Brace bottom chord with approved sheathing or purlins per Bracing Summary.
- Lateral bracing shown is for illustration purposes only and may be placed on either edge of truss member.
- A creep factor of 1.50 has been applied for this truss analysis.
- Listed wind uplift reactions based on MWFRS & C&C loading.



ALL PERSONS FABRICATING, HANDLING, ERECTING OR INSTALLING ANY TRUSS BASED UPON THIS TRUSS DESIGN DRAWING ARE INSTRUCTED TO REFER TO ALL OF THE INSTRUCTIONS, LIMITATIONS AND QUALIFICATIONS SET FORTH IN THE EAGLE METAL PRODUCT'S DESIGN NOTES ISSUED WITH THIS DESIGN AND AVAILABLE FROM EAGLE UPON REQUEST. DESIGN VALID ONLY WHEN EAGLE METAL CONNECTORS ARE USED.

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